

Answer all Questions

Q1 (50 %): Check the two way shear action (punching shear) only around a corner column (450×450) mm in a flat plate floor of a span (7.5×7.5) m. Find the area of vertical shear reinforcement if required. Assume $d = 140$ mm. Total $q_u = 15$ kPa (including slab weight), $f_c' = 28$ MPa, $f_y = 414$ MPa.

Q2 (50 %): for the transverse interior frame (A) of the flat plate floor shown in figure below by using direct design method find:

1. Longitudinal distribution of the static moment at factored loads.
2. Lateral distribution of the moment at exterior support.

Slab thickness = 170 mm, $d = 144$ mm

$q_u = 35.0$ kN/m²

All edge beams = 350 × 700 mm

All columns = 400 × 400 mm

$f_c' = 25$ MPa, $f_y = 420$ MPa

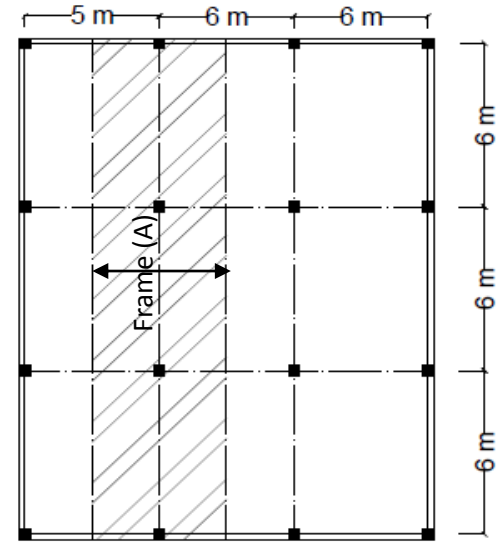


Table 8.10.4.2—Distribution coefficients for end spans

	Exterior edge unrestrained	Slab with beams between all supports	Slab without beams between interior supports		Exterior edge fully restrained
			Without edge beam	With edge beam	
Interior negative	0.75	0.70	0.70	0.70	0.65
Positive	0.63	0.57	0.52	0.50	0.35
Exterior negative	0	0.16	0.26	0.30	0.65

$$C = \Sigma \left(1 - 0.63 \frac{x}{y} \right) \frac{x^3 y}{3} \quad \beta_t = \frac{E_{cb} C}{2E_{cs} I_s}$$

Table 8.10.5.1—Portion of interior negative M_u in column strip

$\alpha_1 \ell_2 / \ell_1$	ℓ_2 / ℓ_1		
	0.5	1.0	2.0
0	0.75	0.75	0.75
≥ 1.0	0.90	0.75	0.45

Note: Linear interpolations shall be made between values shown.

Table 8.10.5.2—Portion of exterior negative M_u in column strip

$\alpha_1 \ell_2 / \ell_1$	β_t	ℓ_2 / ℓ_1		
		0.5	1.0	2.0
0	0	1.0	1.0	1.0
	≥ 2.5	0.75	0.75	0.75
≥ 1.0	0	1.0	1.0	1.0
	≥ 2.5	0.90	0.75	0.45

Note: Linear interpolations shall be made between values shown. β_t is calculated using Eq. (8.10.5.2a), where C is calculated using Eq. (8.10.5.2b).

Table 8.10.5.5—Portion of positive M_u in column strip

$\alpha_1 \ell_2 / \ell_1$	ℓ_2 / ℓ_1		
	0.5	1.0	2.0
0	0.60	0.60	0.60
≥ 1.0	0.90	0.75	0.45

Note: Linear interpolations shall be made between values shown.

v_c with non shear reinforcement is least of	$0.33 \lambda \sqrt{f_c'}$
	$0.17 \left(1 + \frac{2}{\beta} \right) \lambda \sqrt{f_c'}$
	$0.083 \left(2 + \frac{\alpha_s d}{b_o} \right) \lambda \sqrt{f_c'}$
v_c with shear reinforcement	$v_c = 0.17 \lambda \sqrt{f_c'}$
Maximum v_u with shear reinforcement	$v_u = \phi 0.5 \lambda \sqrt{f_c'}$
Shear resist by reinforcement	$v_s = \frac{v_u}{\phi} - v_c = \frac{A_v f_y}{b_o s}$

Note: β is the ratio of long side to short side of the column
 $\lambda = 1$ for normal concrete, $\alpha_s = 40$ for interior column,
 $\alpha_c = 30$ for edge column & $\alpha_c = 20$ for corner column